

Strengthening Of RC Beams Using Glass Fibre Reinforced Polymer Sheets And Comparison Of U Wrap And 90 Degree Strip Wrap.

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ABSTRACT:

Strengthening of reinforced concrete beams with externally bonded fiber reinforced composites is a technique that has been developed in recent years. Here in the present study Glass Fiber reinforced polymer sheets are used for finding behavior of beams strengthened with composite materials. Eight beams are casted, two beams as control beams. Resin mortar with promoter, catalyst, and accelerator, is used as a binding material in various patterns. After 24 hours of wrapping and 7 days air curing beams are tested. U wrapping, Bottom wrapping, 45 degree U wrapping, 90 degree U wrapping, Combination of Bottom wrapping with 90 degree U wrapping are tested in static three point loading frame set up. The test results were evaluated in terms of load deflection behavior, ultimate load carrying capacity, ultimate deflection, crack patterns and associated failure modes. The results obtained clearly demonstrate the effectiveness of strengthening of RC beams using Glass reinforced polymer sheets. The beams treated with Resin mortar with accelerator, catalyst and promoter improved the strength and load carrying capacity.

Keywords: Glass Fiber reinforced Polymer (GFRP), Strengthening, Retrofitting Three Point Loading Frame, Catalyst, Promoter, and Accelerator, Load Carrying Capacity, Wrapping.

I. INTRODUCTION

Reinforced Concrete (RC) structures have been one of the major structural materials for over a century and are still the most popular material for public structures all over the world. Reinforced concrete beams are structural elements designed to carry transverse external loads. These loads cause bending moment, shear forces and torsion across their length in some cases. Concrete is strong in compression and very weak in tension. Thus, steel reinforcement is used to take up tensile stresses in RC beams. In recent years, the field of concrete structure strengthening has become a hot point. As a result, the related strengthening techniques of concrete structure have been an important research field in structural engineering. The use of composites for strengthening and repairing RC structures has gained importance in civil engineering. Strengthening of reinforced concrete structures with externally bonded fiber reinforced polymer (FRP) composites is a newly developed technique in recent years. Generally, FRP strengthened RC beams consist of four materials: concrete, steel bars, adhesives, and FRP reinforcement. Benefits of FRP composites include light weight, high strength and high modulus, durability and impact resistance. FRP's structural properties are useful in absorbing seismic or blast energy, and this property lets the material to act as a polymer damper at flooring area and connection zone. These are

isuccessfully implemented into enhance the performance of structural elements in flexure, axial, shear, and torsion. The commonly used FRP has some drawbacks like debonding of FRP from the concrete, poor behaviour of epoxy at high temperature, inability to apply on wet surface, relatively high cost, etc. and these can be solved by using Glass, fibers, Basalt fiber, Nylons etc.

II. MATERIALS USED

2.1 CONCRETE

Concrete is a construction material of Portland cement and water combined with sand, gravel, crushed stone, or other inert material such as expanded slag or vermiculite. The cement and water form a paste which hardens by chemical reaction into a strong, stone-like mass. The quality of the paste formed by the cement and water largely determines the character of the concrete. Proportioning of the ingredients of concrete is referred to as designing the mixture, and for most structural work the concrete is designed compressive strengths of 15 to 35 MPa. Pozzolona Portland cement will be used. Ordinary clean portable water free

from suspended particles and chemicals will be used for both mixing and curing of concrete.

2.2 REINFORCEMENT

The longitudinal reinforcements used were high-yield strength deformed bars of 10mm diameter and 10 mm diameter were used as hanger bars. The stirrups were made from mild steel bars with 8mm diameter.

2.3 GLASS FIBRE REINFORCED SHEETS

Glass fiber reinforced polymer (GFRP) is a composite construction material resulting from the combination of unsaturated polyester based resin used as a binder with glass fiber. The fibers may be randomly arranged, flattened into a sheet (called a chopped strand mat). These are fibers commonly used in the naval and industrial fields to produce composites of medium-high performance. Their peculiar characteristic is high strength. Glass is made up of silicon (SiO_2) with a tetrahedral structure (SiO_4).



Fig 1.1: Glass Fiber Sheet

Material characteristics	Glass fiber reinforced polymer sheets
Density (g/cc)	2.60
Tensile Strength (MPa)	2050
Elastic Modulus	85

Table 1.1: Properties of Glass fiber reinforced polymer sheets.

The major advantages of glass fibers are:

- Cheaper and more flexible than carbon fiber
- Stronger than many metals by weight
- Non-magnetic and non-conductive
- Highly flexible and can be moulded into complex shapes
- Chemically inert under many circumstances
- Inherent strength
- Weather-resistant finish
- Thermal resistant

2.2 MORTAR RESIN

Epoxy Mortar is a polymer based bonding paste that comprises materials such as epoxy resin (Vinyl Ester), hardener (Cobalt Octoate), catalyst (MEKP-Ketone Peroxide) and a Promoter. It is used to bind the glass fiber sheets with concrete specimens in the form of coatings for resisting debonding failures. The compressive strength of resin mortar is lower than ordinary Portland cement. The toughness of epoxy resin is better than ordinary Portland cement.



Fig.2.1: Resin mortar with catalyst, hardener and catalyst

III. RETROFITTING OF BEAMS

Before bonding the composite fabric onto the concrete surface, the required region of concrete surface was made rough using a coarse sand paper texture and cleaned with an air blower to remove all dirt and debris. Then apply cement mortar on cracked surfaces to fill cracks formed due to axial loading. Once the surface was prepared to the required standard, the epoxy resin was mixed. Mixing was carried out in a plastic container and was continued until

the mixture was in uniform colour. When this was completed and the fabrics had been cut to size, the epoxy resin was applied to the concrete surface. The composite fabric was then placed on top of epoxy resin coating and the resin was squeezed through the roving of the fabric. This operation was carried out at room temperature. Concrete beams strengthened with glass fiber fabric were cured for 48 hours at room temperature before testing.



Fig 3.1: Application and fixing of glass fibre sheet and resin mortar



3.2: iiU iiwrapping iusing iiglass iifiber iisheets



Fig ii3.3: ii90 iidegree iiwrapping iusing iiglass iifiber iisheets



Fig ii3.4: iiCombination ii of ii45degree ii and ii90 ii degree ii strip ii wrapping ii using ii glass ii fiber ii sheetsii (white ii cement ii applied)

IV. TWO POINT LOADING

In ii two ii point ii loading ii the ii load ii is ii transmitted ii through ii a ii load ii cell ii and ii spherical ii seating ii on ii to ii a ii spreader ii beam.ii This ii beam ii bears ii on ii rollers ii seated ii on ii steel ii plates ii bedded ii on ii the ii test ii member ii with ii mortar, ii high- ii strength ii plaster ii or ii some ii similar ii spreader ii plates.ii The ii loading ii frame ii must ii be ii capable ii of

ii carrying ii the ii expected ii test ii loads ii without ii significant ii distortion.ii Ease ii of ii access ii to ii the ii middle ii third ii for ii crack ii observations, ii deflection ii readings ii and ii possibly ii load ii corresponding ii to ii each ii deflection ii is ii noted.ii Crack ii patterns ii are ii marked ii with ii different ii colour ii pens ii when ii formed ii at ii failure,

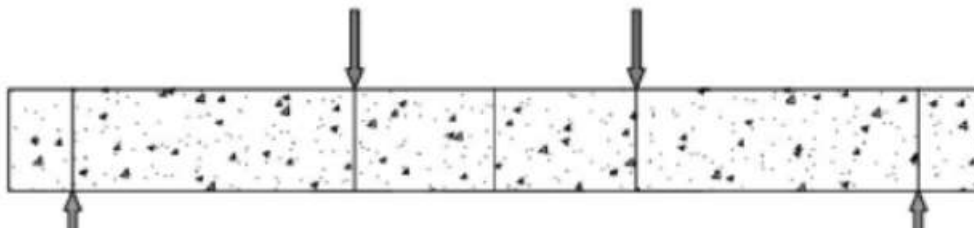


Fig ii4.1: iiTwo ii point ii loading ii of ii beams



Fig ii4.2: iiShear ii force ii diagram



Fig ii4.3: iiBending iimoment iidiagram



Fig ii4.4: iiExperimental iiset iup iifor iibeams

4.iiPROJECT iiPROCEDURES

4.1 iiCube iicompressive iistrength

Casting iiof iiconcrete iispecimens iiis iidone iias iiper iiIndian iiStandards.iiM20 iimix iiis iichosen iiand iidone iimix iidesign.iiMix iiratio iiobtained iiis ii0.47: ii1: ii1.67: ii2.79.iiCompressive iistrength iiof iiconcrete iiis iidetermined iiby iimaking iicubes iiof iisize ii150 iimm iix ii150 iimm.iiCubes iiare iimade iiby iifinding iout iithe iirequired iiamount iiof iiquantities iiof iimaterials iusing iimix iiproportion.iiMixing iiof iiconcrete iiis iicarried iout iimanually.iiCompressive iistrength iiis iithe iicapacity iiof iia iimaterial iior iithe iiability iiof iia iistruure iito iwithstand iiloa iitending iito iireduce iisize.iiTotally ii15 iicubes iiwere iicasted

iifor iidetermination iiof iicompressive iistrength.iiAfter ii24 iihours iithe iimould iiwere iidemoulded iiand iisubjected iito iewater iicuring.iiBefore iitestng iithe iicubes iiwere iidried iifor ii2 iihours.iiAll iithe iicubes iiwere iitested iin iisaturated iiconditions iiafter iiwiping iout iisurface iimoisture.iiThe iiloa iibas iiapplied iwithout iishock iiand iincreased icontinuouslyuntil iithe iiresistance iiof iithe iispecimen iito iithe iincreasing iiloa iibreaks iidown iiand iino iigreater iiloa iican iibe iisustained.iiThe iimaximum iiloa iiapplied iito iithe iispecimen iibas iithen iirecorded; iithree iicubes iieach iiwere iitested iiat iithe iiage ii7 iidays iiand ii28 iidays iiof iicuring iifor iiconcrete iicompression iitestng.



Fig ii4.1.1: iiUnmoulded iispecimen iifor itesting



Fig 4.1.2: Compression test for cube specimen

Table ii4.1.1: iiCompression itest iivalues

Cube iiNo.	C/S ii(mm ²)	7 iiday's iiStrength ii(N/mm ²)	28 iiday's iiStrength ii(N/mm ²)
Cube ii1	150 iix ii150	14.66	28.88
Cube ii2	150 iix ii150	15.11	28.88
Cube ii3	150 iix ii150	15.12	30.22
Average		14.96	29.47

4.2 TESTING OF CYLINDERS

Casting of concrete specimens is done as per Indian Standards. M20 mix is chosen and done in mix design. Mix ratio obtained is 0.47: 1: 1.34: 2.29. Compressive strength of cylinders of size 150 mm x 300 mm x 150 mm is determined. Cylinders are made by finding out the required amount of quantities of materials using mix proportion. Mixing of concrete is carried out manually.

First the coarse aggregate and fine aggregate are mixed. After that the cement is poured into the mixer. Required amount of water is added. And the resulting

concrete with uniform appearance is transferred to moulds. In assembling the mould for use, the joints between the sections of mould is thinly coated with oil and a similar coating of oil is applied between the contact surfaces of the bottom of the mould and the base plate in order to ensure that no water escapes during the filling. The interior surfaces of the assembled mould are thinly coated with oil to prevent adhesion of the concrete. After 24 hours of air curing the specimens are transferred to the curing tank.



Fig 4.2.1: Testing of cylinder specimens

Table 4.2.1: Tensile strength of specimens

Cube No.	C/S (mm ²)	28 Day Strength (N/mm ²)
1	150 x 300	2.83
2	150 x 300	2.97
Average		2.9

4.3 SPECIMEN PREPARATION AND TESTING

Form work making use of plywood was prepared for the beam of size 1700mm x 150 mm x 200 mm size. A total of 8 beams were cast where in 2 were controlled specimens and 2 were subjected to U-wrapping and other 2 specimens were subjected to 90 degree strip wrapping and 2 were subjected to 45 degree strip wrapping. Each of the specimens were singly reinforced and under reinforced section. Without delay after the beam cast, the beams were

covered with plastic sheet to minimize the evaporation of water from the surface of the beam specimen. After 24 hours, the sides of the formwork were removed and the beams were lowered into a curing tank for 28 days, after which the beams were left alone until the time of test.

Before testing, beams were whitewashed and then the surface was rubbed with sand paper and tested in two points loading with a maximum capacity of 30 tons. The beam was placed over the two steel rollers bearings leaving 75 mm from the both sides of

iiRest iiof iithe iipart iawas iiequally iidedived iin iito iithree iiequal iiparts.iiLoad iawas iiapplied iiby iiloading iicell iiof ii1000 iikN.iiTwo iidial iigauges iwere iiused iifor iirecording iideflection.iiOne iidial iigauge iawas iiplaced iiat iicenter iand iiother iawas iiplaced iiunder iithe iione iiof iithe iipoint iiload iito iinote iithe

iideflection.iiBeams iwere iitested iibefore iand iiafter iiretrofitting.iiFirst iiof iiall iicontrol iibeam iawas iitested iewith iifull iiload iito iiget iithe iimaximum icollapse iiload, iithen iiafter iiother iibeams iwere iitested iewith iiload iiof ii75% iiof icollapse iiload.



Fig ii4.3.1: iiTesting iiof iibeams iibefore iiretrofitting iand iininitial icracks iiare iimarked



Fig ii4.3.2: iiCracked iispecimen iimarked ii



Fig ii4.3.3: iiTesting iiof iibeams iiafter iiretrofitting iiwith iiwrapped iiglass iifiber iisheets



Fig ii4.3.3: iiCracked iibeams iiafter iiretrofitting

4.4 iiWRAPPING iiPATTERNS ii

Totally iiwe iihave iisix iibeams iifrom iiwhich ii2 iifor iiU iiwrapping, ii2 iifor ii45 iidegree iistrip iiwrapping iiland ii2 iifor ii90 iidegree iistrip iiwrapping.iiHere iiU iiwrapping iiland ii90 iidegree iistrip iiwrapping iiare iiconsidered iifor iithe iistudy.90 iidegree iiland iiU iiwrapping iiare iithe iitwo iipatterns iiof iiwrapping iibeams iiusing iiglass iifiber iisheets.iiFor iithe ii90 iidegree iistrip, iiwrapping iithe iiglass iifiber iisheet iis iicut iiin iito iistrips iiof ii5mm iipieces iiland iipasted iion iithe iisurface iiof iieffective iilength iiof iicollapsed iibeam iiin ii5mm iiof iispacing iito iisave iithe iicost iiof iimaterials.iiIt iirequires iionly ii0.36 iisquare iimeter iiof iiglass iifiber iito iiwrap iione iibeam iispecimen.

Strips iiare iipasted iion iithe iibeam iiusing iiresin iimortar iiby iiflat iiblade iiin ii90 iidegree iiU iishaped iixcluding iitop iisurface iiland iikeep iiwrapped iibeams iiin iiroom iitemperature iifor ii48 iihours iiland iitested.iiFor iiU iiwrapping iiglass iifiber iisheet iis iicut iiin iito iishape iiof iiU iiland iis iipasted iion iithree iisides iiof iieffective iilength iiof iicollapsed iibeam iiusing iiresin iimortar iias iibonding iiaagent.iiFor iiU iiwrapped iibeam iit iirequires iione iisquare iimeter iiof iiglass iifiber iito iiwrap iithe iibeam iispecimen iiAfter ii48 iihours iikeeping iispecimens iiat iiroom iitemperature, iitwo iispecimens iiof iiU iiwrapped iiland iitwo iispecimens iiof ii90 iidegree iistrip iiwrapped iibeams iiare iitested iiin iitwo iipoint iiloading iiframe iiland iicompared iithe iiresults iiby iitaking iibest iiof iitwo iireadings.



Fig ii4.4.1:90 iidegree iiwrapped iicollapsed iibeam iifor iiretrofitting

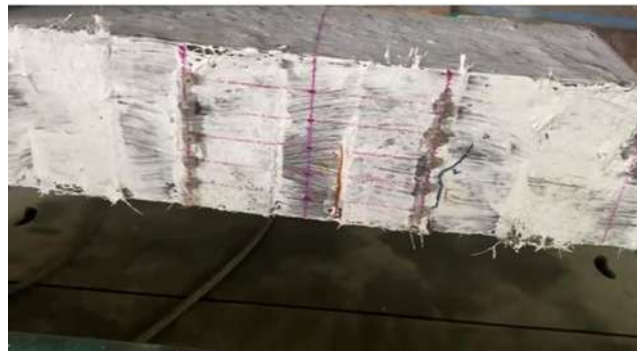


Fig ii4.4.2: iiCracked iipatterns ion ii90 iidegree iiwrapped iicollapsed iibeam iiafter iiloading



Fig ii4.4.3: iiU iiwrapped iicollapsed iibeam iifor iiretrofitting



Fig ii4.4.4: iiCracked iipatterns ion iiU iiwrapped iicollapsed iibeam iiafter iiloading

4.5 iiGRAPHICAL iiRESULTS

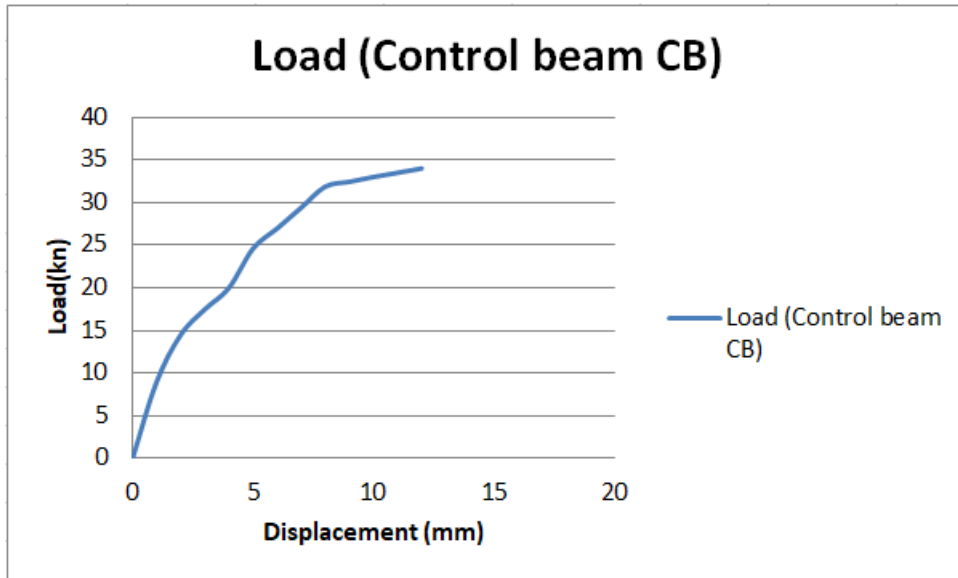


Fig 4.5.1: Load v/s Displacement graph of control beam (CB)

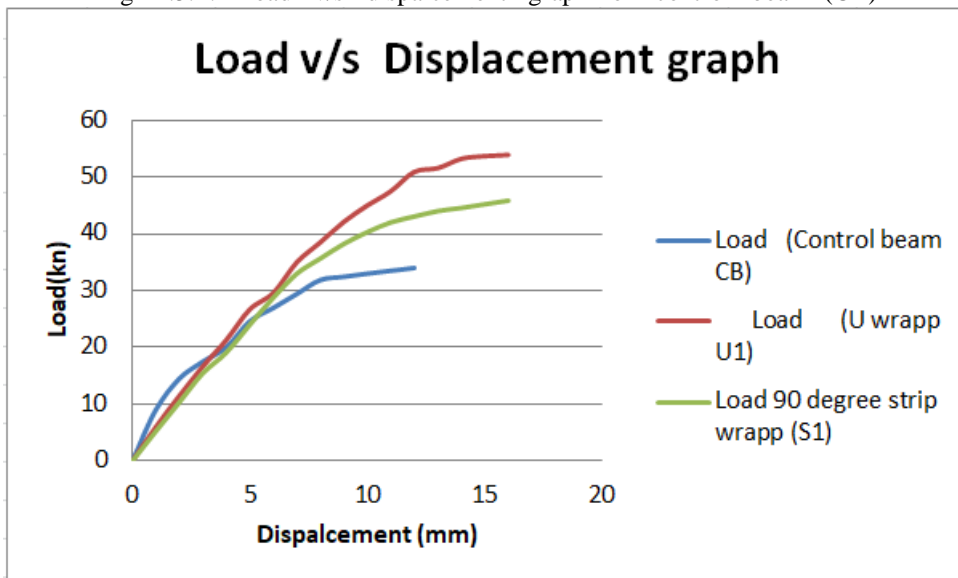


Fig 4.5.2: Load v/s Displacement graph of U wrap beam (U1) v/s 90 degree strip wrap beam (S1)

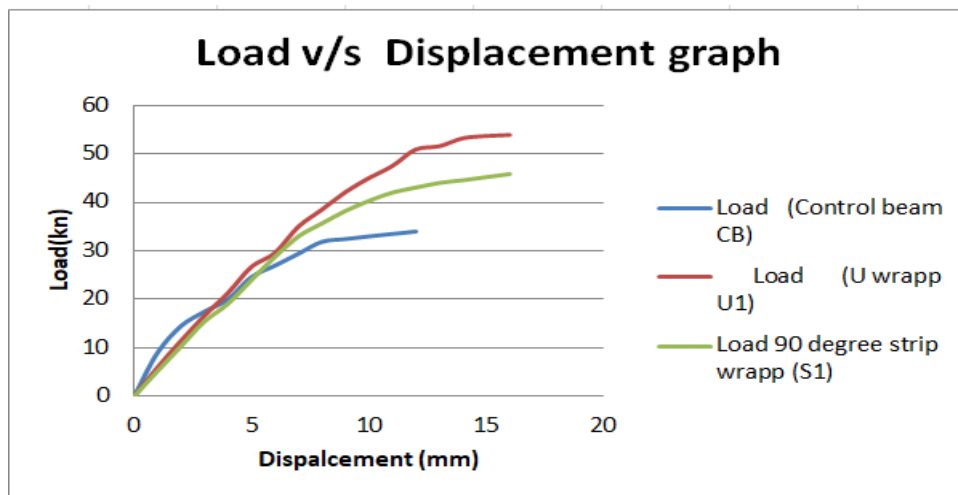


Fig ii4.5.3: iiLoad iiv/s iidisplacement iigraph iiof iiControl iibeam iiCB iiv/s iiU iwrap iibeamii(U1) iiv/s ii90 iidegree iistrip iwrap iibeam ii(S1)

4.6: iiCOMPARITIVE iiSTUDY

Here iiwe iitaken iithree iibeams iifor iithe iicomparative iistudy.1.non iiretrofitted iicontrol iibeam ii(CB) ii2.retrofitted iibeam iiby iiU iwrap ii(U1) ii3.retrofitted iibeam iiby ii90 iidegree iwrap ii(S1).Control iibeam iis iithe iimother iibeam iwhich iis iifully iicollapsed iewith iia iultimate iiload iicapacity iiof ii34 iikN iand iiother iitwo iibeams iiare iiloaded iiby ii75

iipcentage iiof iicollapsed i.Here iive iitaken iibest iione iiof iitwo iireadings iion iieach iirapping iistyles.iiAfter iiretrofitting iione iiof iipartially iiloaded iibeam iispecimen iiby iiU iwrap iusing iiglass iifiber iion iieffective iilength iexcluding iitop iilayer iive iiget iimaximum iiload iicarrying iicapacity iiof ii54 iikN iand ii90 iidegree iistrip iwrapped iibeam iiby ii46 iikN.

Table ii4.6.1: iiLoad iicarrying iicapacity iiof iiCB, iiU1&S1

SPECIMEN	LOAD iicARRYING iicapacity(kN)
Control iibeam ii(CB)	34
Beam ii1 ii(U1)	54
Beam ii2 ii(S1)	46

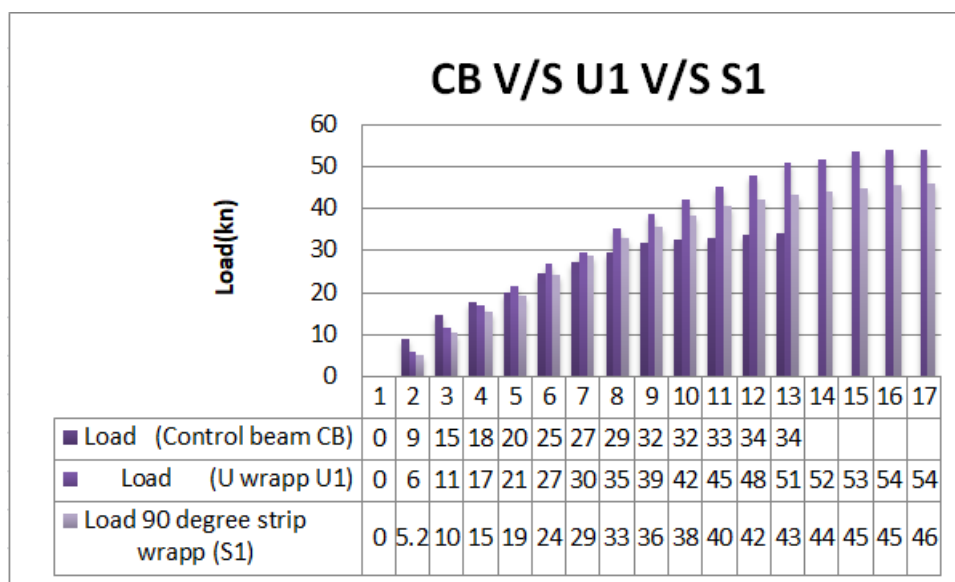


Fig ii4.6.1: iiBar iigraph iiof iiCB, iiU1 & iiS1

V. CONCLUSION

A itotal iiof iisix iibeams iiwere iicast iiout iiof iiwhich iitwo iiwere iicontrolled iibeams iiaand iitwo iiwere iiretrofitted iior iiwrapped iiby iiU iwrap iiaand iithe iiother iitwo iiby ii90 iidegree iistrip iiaand iitaken iibest iivalue iiof iitwo iireadings iiafter iiappling ii75% iiof iithe iiUltimate iiLoad iiNo iihorizontal iicracks iiwere iiobserved iiat iithe iilevel iiof iithe iireinforcement, iiwwhich iindicated iithat iithere iiwere iino iioccurrences iiof iibond iifailure.iiOther iimportant iiconclusions iiare iias iifollows:

- The iiload iicarrying iicapacity iiobtained iitwo iipatterns iiare iigood iiaand iican iibe iiconsidered iias iigood iisolution iifor iistrengthening iiof iirc iicollapsed iibeams.
- There iis iiconsiderably iincrease iin iiload iicarrying iicapacity iiof iiU iwrapped iibeam iiby ii20% ii compared iiaand ii90 iidegree iiwrapped iibeam iiby ii12 ii% iicompared iito iicontrol iibeam.
- The iiload iicarrying iicapacity iiof iibeams iiwrapped iiby iiU iwrap iis iilargeri(54kN) iias iicompared iito iithe ii90 iidegree iiwrapped iibeam iifor iistrengthening iior iiretrofitting.
- As iieconomical iipoint iiof iiview iieach iiU iwrapped iibeam iicovers iian iiaarea iiof ii0.36 imeter iiglass iifiber imaterial iis iivery iilarge iias iicompared iewith ii90 iidegree iiU iwrapped iibeam.
- So iit iis iixperimentally iiproved iithat ii, iif iiU iwrapped iibeam ihave iiload iicarrying iicapacity iiof ii54 ikN iiby iiconsuming iione iisquare imeter iiglass iifiber iiaand ii90 iidegree iiwrapped iibeam iiof ii46 ikN iiby iiconsuming iivery iiless iiaarea iiof imaterial iie ii0.36, ii90 iidegree iwrap iipattern iis iigood iiaand iieconomical iifor iistrengthening iiof iicollapsed iibeam.
- Control iibeam iilos iinitial iistiffness iiwhile iicollapsed, iibut iawe iiwrapped iior iiretrofitted iicollapsed iibeams iit iigains iisome iistiffness iiaand iidhows iiductility iisufficient iibehaviour iias iicompared iito iicontrol iibeam

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The iiauthors iiwould iilike iito iiacknowledge iiGod iiAlmighty iifor iHis iieternal iigrace iiaand iiguidance iias iiwel iiEr iiNithiniiMohan., iiPHD

iiScholar, iiIT iiBombay, iifor iiall iihis iamazing iiguidance iiaand iisupport.

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